

## **On ice pile-up height at the front of structure due to tsunami with sea ice**

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### **ABSTRACT**

There is much potential for damage due to tsunami with sea ice since megaquake in the vicinity of Chishima Trench, where locates at the south-eastern part of Hokkaido, Japan could happen any time. Though return period of earthquake here is from 340 to 380 years, more than 400 years have passed since the most recent major earthquake. In addition, on the 4<sup>th</sup> of March 1952, the Tokachi-oki earthquake (M8.2) occurred in the south-eastern part of Hokkaido, Japan. It created a tsunami 1-2m high on the eastern coast of Hokkaido, which, together with remaining sea ice, caused damage to residences and infrastructure. This action is likely to close roads and isolate settlements. A disaster prevention plan for opening roads to stricken areas is important for transport of goods and people. Therefore, it is crucial to understand the impact of sea ice on road structures when a tsunami occurs in a sea-ice-infested area. The purpose of this research is to give a useful information on the interaction between sea ice and structures to help road opening plan. More than 200 indoor-model tests have been conducted to determine the interaction mechanism between sea ice and road structures using models of sea ice made of poly-propylene. A method for estimating sea ice pile-up height at the front of a road structure is proposed here referring to Kovacs and Sodhi (1979). It is found that ice pile-up height is influenced by especially angle of repose of sea ice piece.

**KEY WORDS:** Ice pile-up; Tsunami; Earthquake

### **INTRODUCTION**

On the 4<sup>th</sup> of March 1952, the Tokachi-oki earthquake (M8.2) occurred in the south-eastern

part of Hokkaido, Japan. It created a tsunami 1-2m high, which, together with remaining sea ice, caused damage to residences and infrastructures. The pieces of sea ice were around 2m square (5m square as maximum) and 0.6m thick (1.3m thick as maximum) (Report of the Tokachi-oki Earthquake, 1954: Kioka, 2013a : Kioka, 2013b). Also, the 2011 Tohoku tsunami caused water to rise due to ice jam that occurred in the river and impact of ice on the water gate (Yoshikawa, 2012). In addition, it is reported that there is much potential for damage due to tsunami with sea ice since megaquake in the vicinity of Chishima Trench, where locates at the south-eastern part of Hokkaido, Japan could happen any time. Though return period of earthquake here is from 340 to 380 years, more than 400 years have passed since the most recent major earthquake. Other damage caused by tsunami run-up with ice floes have been reported (Kioka, 2015: Kioka, 2016). This action is likely to close roads and isolate settlements. However, there has been little research reported on the interaction between sea ice and road structures. Opening roads to stricken areas is important for transportation route for relief goods and people. This requires elimination of roads obstacles. Therefore, it is crucial to understand the interaction between sea ice and road structures when a tsunami occurs in a sea-ice-infested area. In the Sea of Okhotsk, sea ice can be seen overtopping breakwaters even during bad weather, as shown in Figure 1. In the present study, 238 indoor-model tests were conducted to determine the interaction mechanism between sea ice by run-up tsunami and road structure. The sea ice shape, size and thickness (11types) in addition to water level difference and road structure height were considered as parameters. Also, the model by Kovacs and Sodhi (1979) was applied to estimate ice pile-up height in front of the road structure.



Figure 1. Sea ice overtopping breakwaters in the Sea of Okhotsk (2015 not Tsunami)

## TEST AND TEST CONDITIONS

Figure 2 shows the experimental device with a scale factor of 1/50, following the similarity rule of Froude. Width and length of channel are 0.4m and 9m, respectively. Ice floes were simulated by disk-shaped pieces and cuboid pieces made of polypropylene (density: 0.912 g/cm<sup>3</sup>) with 11 types of size  $D$  and thicknesses  $t$  shown in Figure 3 and Table 1.

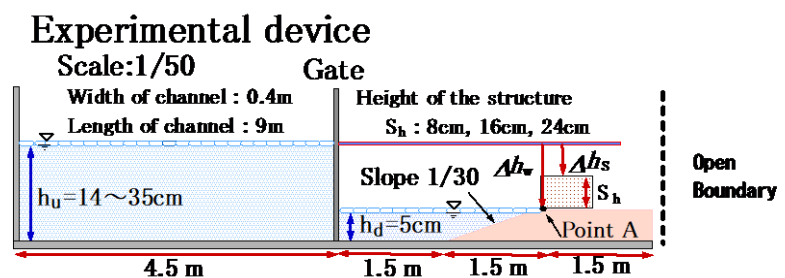


Figure 2. Experimental device

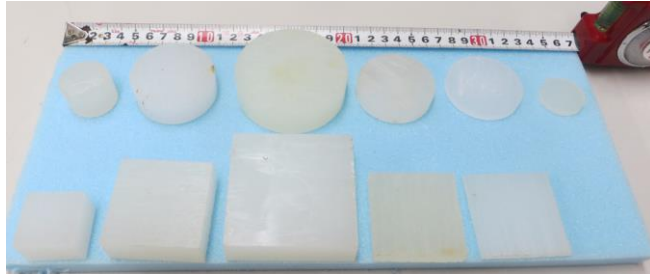


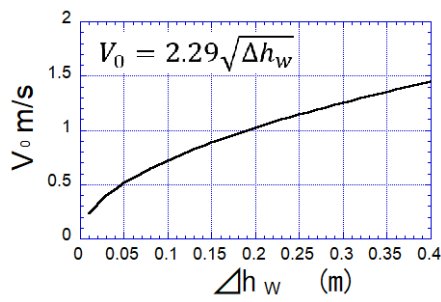
Figure 3. Polypropylene (11types)

A tsunami was then created by opening the gate suddenly. The sizes of the model sea ice at actual scale correspond to 1.5, 2.5 and 3.5m and the thickness correspond to 0.25m, 0.5m and 1m. Upstream water depth  $h_u$  was varied from 14cm to 38cm, and downstream water depth  $h_d$  and sea-bed slope were kept at 5cm and 1/30, respectively.

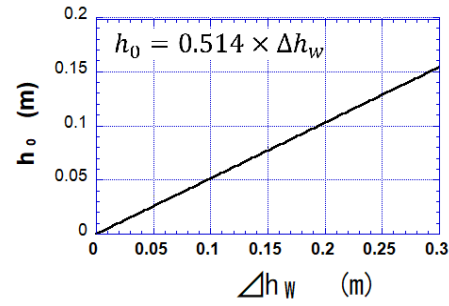
Table 1. Ice floe model cases

| $D$ cm | $t$ cm | Shape |
|--------|--------|-------|
| 3      | 2      | CP    |
| 5      | 2      | CP    |
| 7      | 2      | CP    |
| 5      | 1      | CP    |
| 5      | 0.5    | CP    |
| 1.5    | 0.5    | DSP   |
| 3      | 2      | DSP   |
| 5      | 2      | DSP   |
| 7      | 2      | DSP   |
| 5      | 1      | DSP   |
| 5      | 0.5    | DSP   |

CP (cuboid pieces)  
DSP (disk-shaped pieces)



(a)



(b)

Figure 4(a)(b). Regression curves for (a) water velocity  $V_0$  and (b) water depth  $h_0$  at Point A.

Model road structure heights  $S_h$  were taken as 8cm, 16cm and 24cm. Dimensions  $\Delta h_w$  and  $\Delta h_s$  were considered as shown in Figure 2. Point A is a reference position where water velocity  $V_0$  and water depth  $h_0$  without the model structure were measured to obtain regression curves before the tests (Figure 4 (a)(b)). Indoor model tests were conducted for 238 cases to determine the interaction mechanism between sea ice caused by run-up tsunami and road structure by videotaping. Note that there was only one sea ice size and shape in each of the 11 test conditions, no mixing of ice pieces in any test.

## TEST RESULTS

From these 238 cases, the interactions between sea ice by run-up tsunami and road structure were observed as shown in Figure 5(a), and the interactions can be divided into 3 types as shown in Figure 5(b), namely, sea ice piled up in front of structure (Type-A), sea ice remaining on structure with sea ice pile-up (Type-B), and sea ice over-topping structure (Type-C).

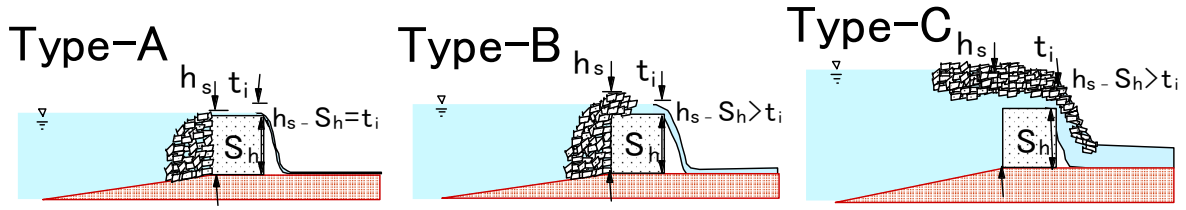


Figure 5(a). Three interaction types (Schematic)

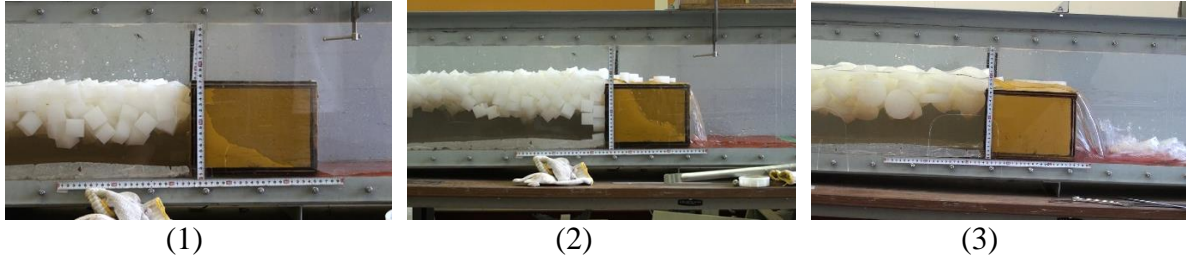


Figure 5(b). Three interaction types (Tests)

- (1) Type-A  $S_h=16\text{cm}$ ,  $\Delta h_w=17\text{cm}$ ,  $\Delta h_s=1\text{cm}$ ,  $D=3\text{cm}$ (cuboid)  $t=2\text{cm}$
- (2) Type-B  $S_h=16\text{cm}$ ,  $\Delta h_w=20\text{cm}$ ,  $\Delta h_s=4\text{cm}$ ,  $D=3\text{cm}$ (cuboid)  $t=2\text{cm}$
- (3) Type-C  $S_h=16\text{cm}$ ,  $\Delta h_w=21\text{cm}$ ,  $\Delta h_s=5\text{cm}$ ,  $D=7\text{cm}$ (disk-shaped)  $t=2\text{cm}$

Pile-up height  $h_s$  against dimension  $\Delta h_w$ , which relates to tsunami height, is investigated through the ice pile up model proposed by Kovacs & Sodhi (1979) (see Figure 6). In the model, the increase in gravitational potential energy for ice pile-up volume is equal to the work done by external force. The conservation of mass inside ice pile-up is also considered, and the following equation was derived.

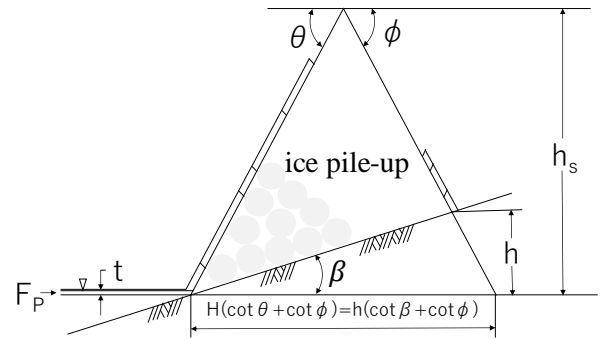


Figure 6. Ice pile-up model (Kovacs & Sodhi, 1979)

$$F_p = \frac{1}{2} \rho_i g h_s t \frac{Q(\theta, \phi, \beta)}{P(\theta, \phi, \beta)} \quad [1]$$

$$P(\theta, \phi, \beta) = \frac{(\tan \theta - \tan \beta)(\tan \theta + \tan \phi)}{(\tan \beta + \tan \phi) \tan^2 \theta} \quad [2]$$

$$Q(\theta, \phi, \beta) = \frac{(\tan \theta + \tan \phi)}{\tan \theta \times \tan \phi} - \frac{(\tan \theta + \tan \phi)^3 \tan^2 \beta}{(\tan \beta + \tan \phi)^2 \tan^3 \theta \tan \phi} \quad [3]$$

where,  $h_s$ : ice pile-up height,  $F_p$ : external force,  $\rho_i$ : ice density,  $t$ : ice floe thickness,  $g$ : gravitational acceleration. Angles  $\theta$ ,  $\beta$  and  $\phi$  are shown in Figure 6.

From Figure 2, the slope of 1/30 corresponds to  $\beta \doteq 1.9$  (deg.), and this gives us

$Q(\theta, \phi, \beta)/P(\theta, \phi, \beta)$  close to 1. Here, coefficient  $C_R$  is introduced to consider only the effect of angle  $\theta$  of repose for simplicity.

Therefore,

$$F_p = \frac{1}{2} C_R \rho_i g h_s t \quad [4]$$

On the other hand, Kioka (2013a) considered this external force  $F_p$  as a drag force  $F_D$  [5].

$$F_p = \frac{1}{2} \rho_w C_D V_0^2 t \quad [5]$$

where,  $\rho_w$ : sea water density,  $C_D$ : drag coefficient,  $V_0$ : water velocity

By using regression curve for  $V_0$  (Figure 4(a)) against  $\Delta h_w$ , and  $C_p = C_D/C_R$ , equating equation [4] and equation [5], yields the following relation [6].

$$h_s = \frac{2.29^2 \rho_w C_p}{\rho_i g} \Delta h_w \quad [6]$$

where,  $C_p$ : coefficient for ice pile-up height.

Using Type-A and Type-B data, the relation between dimension  $\Delta h_w$  and ice pile up height  $h_s$  is plotted in Figure 7(a)-(k). Coefficients  $C_p$ , which are related to the slope of the line in Figure 7(a)-(k), are found to be from 1.65 to 1.9 depending on various shape, size and thickness.

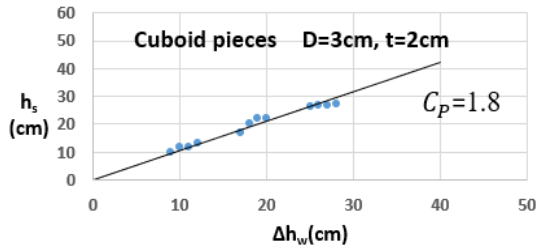
Next, Figure 8(a)-(k) show the plots of interaction types on the plane of  $\Delta h_w$  and  $\Delta h_s$ . In order to examine the transition from the interaction Type A to Type-B, ice pile up height  $h_s$  is compared to structure height  $S_h (= \Delta h_w - \Delta h_s)$  with an additional height  $t_i$ , which is introduced here and defined as  $(h_s - S_h)$  satisfying Type A.

Using also equation [6] and  $S_h (= \Delta h_w - \Delta h_s)$ , equation [7] is obtained.

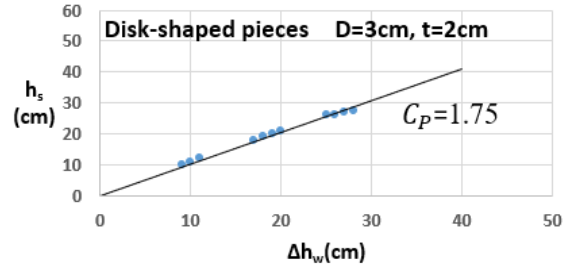
$$\Delta h_s = \left[ 1 - \frac{2.29^2 \rho_w C_p}{\rho_i g} \right] \Delta h_w + t_i = k \times \Delta h_w + t_i \quad [7]$$

where,  $t_i$ : additional height,  $k$ : constant depends on  $C_p$ .

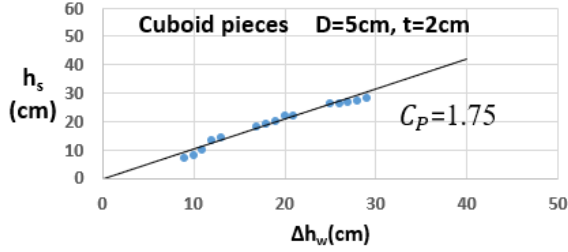
A maximum height of  $(h_s - S_h)$  satisfying Type A is taken here as  $t_i=3\text{cm}$  in Figure 8(a)-(k) to determine line of boundary between Type-A and Type-B. And, using  $\rho_i=912[\text{kg/m}^3]$ ,  $g=9.8[\text{m/s}^2]$ ,  $\rho_w=1000[\text{kg/m}^3]$ , constant  $k=0.032, -0.003, -0.027, -0.056, -0.085, \text{ and } -0.115$  are obtained depending on  $C_p=1.65, 1.7, 1.75, 1.85, 1.9$ , respectively. These equations give the lower boundary of Type-B&C as reference lines. Note that  $\Delta h_w$  is greater than 8cm here since  $\Delta h_w > S_h$ .



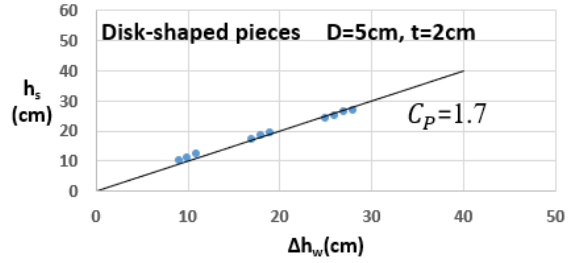
(a) ( $D=3\text{cm}$ ,  $t=2\text{cm}$ ) Cuboid pieces



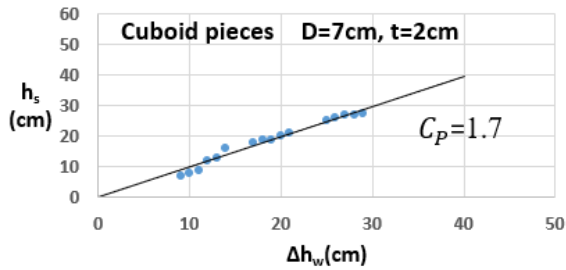
(g) ( $D=3\text{cm}$ ,  $t=2\text{cm}$ ) Disk-shaped pieces



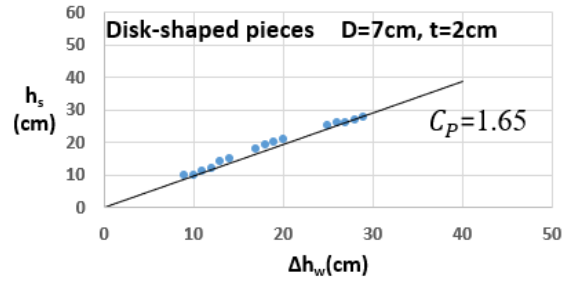
(b) ( $D=5\text{cm}$ ,  $t=2\text{cm}$ ) Cuboid pieces



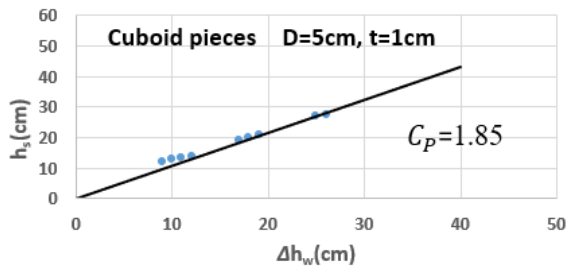
(h) ( $D=5\text{cm}$ ,  $t=2\text{cm}$ ) Disk-shaped pieces



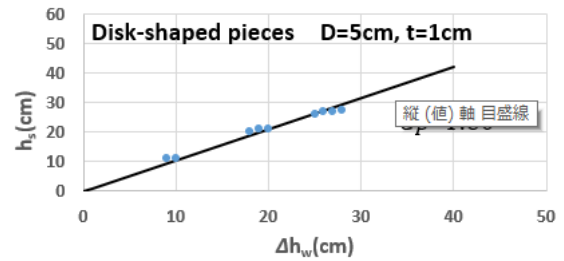
(c) ( $D=7\text{cm}$ ,  $t=2\text{cm}$ ) Cuboid pieces



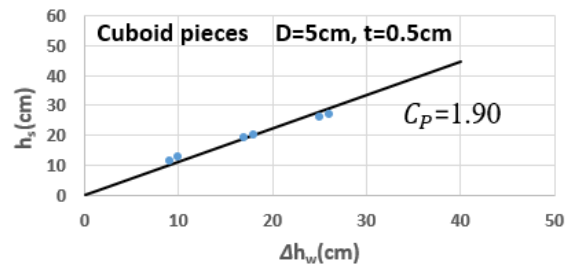
(i) ( $D=7\text{cm}$ ,  $t=2\text{cm}$ ) Disk-shaped pieces



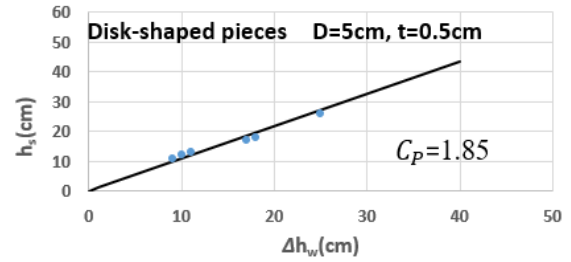
(d) ( $D=5\text{cm}$ ,  $t=1\text{cm}$ ) Cuboid pieces



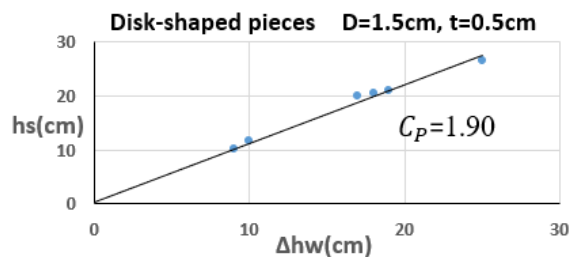
(j) ( $D=5\text{cm}$ ,  $t=1\text{cm}$ ) Disk-shaped pieces



(e) ( $D=5\text{cm}$ ,  $t=0.5\text{cm}$ ) Cuboid pieces



(k) ( $D=5\text{cm}$ ,  $t=0.5\text{cm}$ ) Disk-shaped pieces



(f) ( $D=1.5\text{cm}$ ,  $t=0.5\text{cm}$ ) Disk-shaped pieces

Figure 7. (a)-(k)  $\Delta h_w$  vs.  $h_s$  for Type-A and Type-B test data.

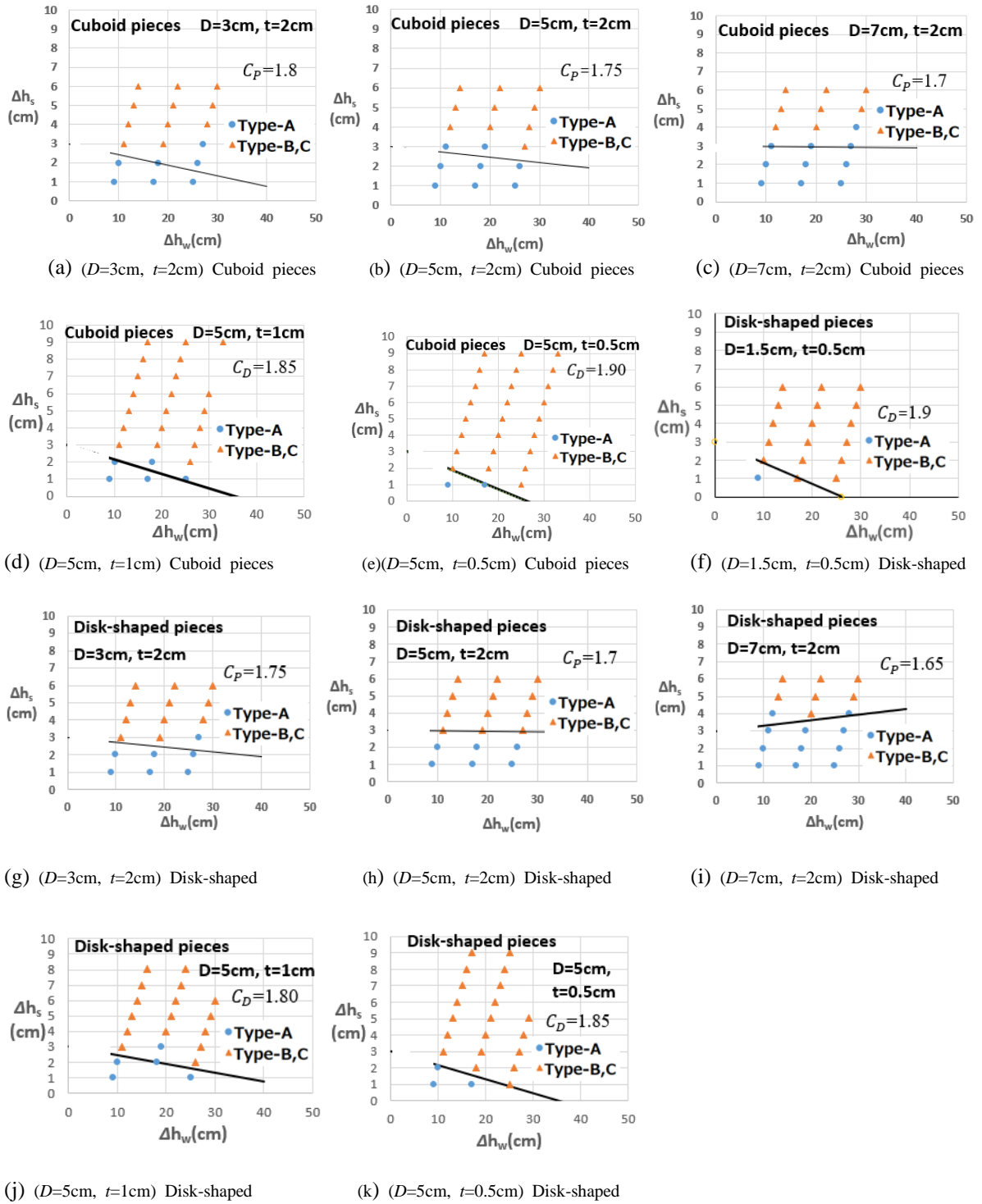


Figure 8. (a)-(k) Interaction types on the plane of  $\Delta h_w$  and  $\Delta h_s$  with line of boundary between Type-A and Type-B.



Also, angle  $\theta$  of repose was measured 5 times for each piece size, and average was estimated. Figure 9 shows coefficient for ice pile up height  $C_P$  vs. angle  $\theta$  of repose. For two types of pieces, as  $C_P$  increases with increasing  $\theta$  and decreasing size and thickness. Note that  $C_D$  depends on Reynolds No, and  $C_D$  of cuboid pieces is greater than that of disk-shaped pieces, for test condition here. So,  $C_P$  depends on size, thickness, shape, and angle  $\theta$  of repose of pieces.

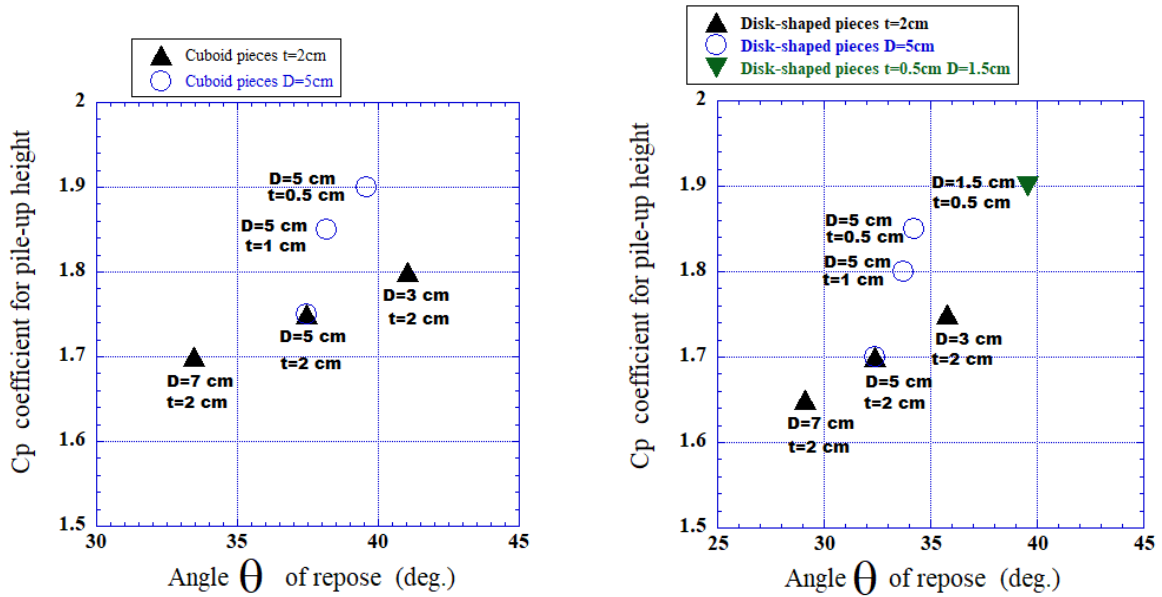


Figure 9.  $\theta$  of repose vs.  $C_P$ . (Cuboid pieces and Disk-shaped pieces)

The following equation [8] is obtained using equation [6] and regression curve for  $h_0$  (Figure 4(b)) against  $\Delta h_w$ .

$$\frac{h_s}{D} = \frac{\rho_w C_P}{\rho_i} \times F_{r0}^2 \times \left(\frac{h_0}{D}\right) \quad [8]$$

where,  $F_{r0}$ : Froude No. at Point A (Figure 2).  $F_{r0}=1.02$  is obtained here from regression curves as shown in Figure 4(a)(b).

Also, test data on  $h_s/D$  is plotted against  $h_0/D$  with reference line for  $C_p=1.9$  as shown in Figure 10. Once  $h_0$  (taken as flood water depth) and  $D$  are given, we can estimate  $h_s$ . So, this figure would be available for a disaster prevention plan for a tsunami in an ice infested sea.

## CONCLUSIONS

The interaction between structure and sea ice by run up tsunami can be divided into three



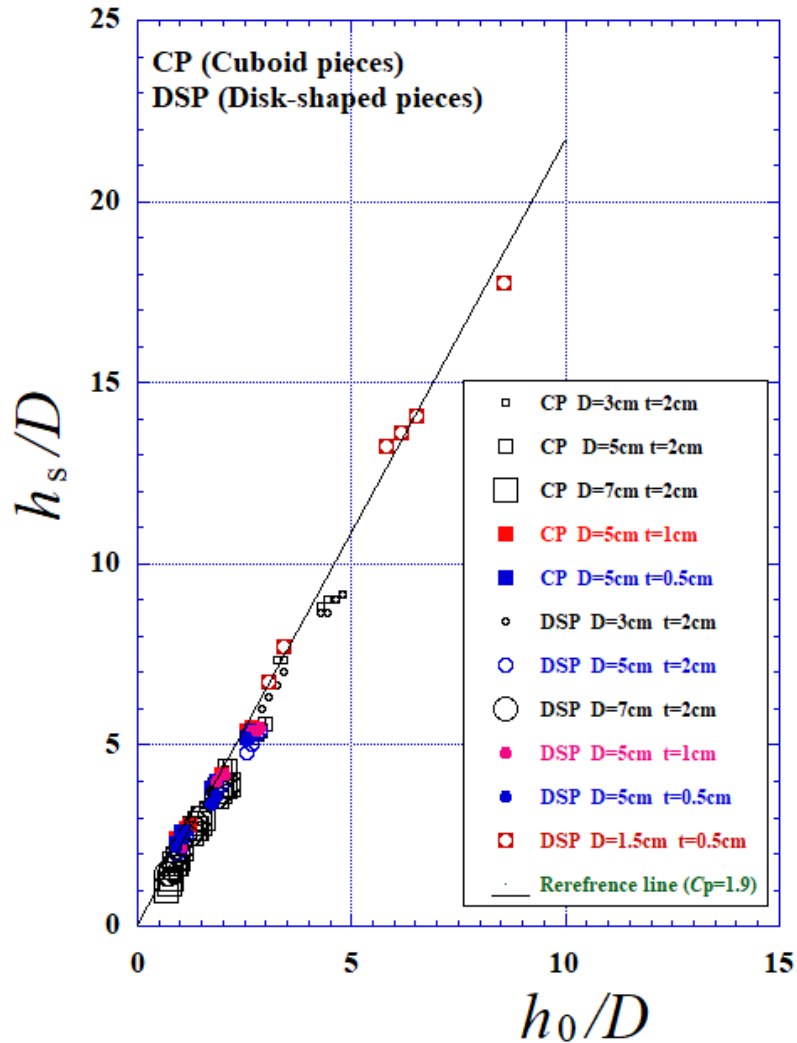


Figure 10.  $h_s/D$  vs.  $h_0/D$

types. The height of sea ice piled up in front of a structure can be estimated from sea water density  $\rho_w$ , sea ice density  $\rho_i$ , coefficient  $C_p$  for ice pile up height and the difference  $\Delta h_w$  between upstream water height and reference height at the bottom of the structure. Once flow depth  $h_0$  and size  $D$  of floe are given, ice piled up height  $h_s$  would be estimated referring Figure 10. These findings will be useful in developing a disaster prevention plan for tsunami in ice infested sea, since elimination of road obstacles is required to rush relief goods and people to the stricken area.

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